





InfoCAD 23 Introductory Examples Finite Elements
Prestressing
Frameworks
Dynamics
BIM

InfoCAD 23

Introductory Examples

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Introductory Examples

This documentation is intended to help you get started with editing beam and shell structures and to demonstrate the various options available for displaying the results. You can press the F1 key to access the online help for the function you are currently using in the program. The help system also contains the complete documentation of the program, including explanations of theoretical principles, calculation methods and results.

During installation example files will be saved to the user directory under *InfoGraph/Samples*. They include projects with beam, cable and shell structures as well as applications relating to structural dynamics and prestressed concrete construction.

The examples are provided without results. To view the results, you first need to perform the relevant calculation. Product news, user tips and updated versions of the program are available at **www.infograph.eu**.

InfoCAD Study Version

The *InfoCAD Study Version* was created as a way to get acquainted with the program and its features by using the examples or your own projects.

The study version provides the same user interface, calculation and checking methods and results output as the full version. The only difference lies in the program capacity, which is limited as follows:

Finite elements:	1000 elements	
	1 area section	
	1 beam section	
Dynamics:	1 eigenmode	
Prestressing:	1 tendon	
Frameworks:	10 beams	
	1 section	

Installation



The program is compatible with Windows 8 or higher. Administrator privileges are required to perform the installation. Only basic user privileges are needed to use the program.

For installation, start the installation file infocad_xxx.exe or infocad_std_xxx.exe for the study version.

In the welcome dialog that appears, select the menu item *Install InfoCAD* and follow the instructions on the screen. You have already received the license number to be entered by e-mail and also used it for the download. This is not required for the study version.

The following applications will be installed and can be launched from the Start menu

- 1. *InfoCAD* or *InfoCAD Study Version* (main program system for static calculations and checks)
- 2. InfoGraph Systemviewer (for system visualization and deformation animation)
- 3. *InfoGraph Crack Width Limitation* (stand-alone program)
- 4. InfoGraph Lateral Torsional Buckling Check (stand-alone program)

To remove the program system, select the *Apps* menu in the Windows *Settings*. There, the automatic uninstall program for InfoCAD can be started. All files created or modified by the user are retained.

General Instructions

Program Start

File name

To start the program, go to the *Programs / InfoGraph* folder in the *Start menu* and select *InfoCAD* or *InfoCAD Study Version*. An empty program window will appear.

Click the *Open* button to access InfoGraph project files with the following extensions (e.g., example files):

.fem	->	Finite Element Project
.rsw	->	3D Frame
.esw	->	2D Frame
.ros	->	Axisymmetric shell

These file types are registered during installation, which means they can be opened by simply double-clicking the file.

Operating Concept

The figure below highlights the most important controls of the *InfoCAD* program interface. For FEM projects, the xy plane appears in the representation area after you select the structure type. Slab and plain stress structures are generally specified in this plane.

Functions can be accessed from a tool bar, a menu item, the dialog bar or the workspace. For graphical functions you normally have to enter additional data, which is then queried in either a separate window or the dialog bar.



Design Codes

The design codes and associated materials available in the program correspond to the regional settings of your computer. If necessary, you can change these settings in the Options menu.



Basic Procedures

Structures are normally specified and calculated using the following work steps:

- 1. Describe the problem with the model objects
- 2. Specify supports and cross-sections
- 3. Generate an element mesh
- 4. Define load cases
- 5. Define the actions and design situations
- 6. Perform calculations
- 7. Display or print out the results

Model objects are critically important for system generation

Model objects describe the geometry and properties of individual structural components. They form the basis for the program-controlled generation of finite element meshes and are accounted for the automatic mesh generators *form-sensitive* and *grid-shaped*.

The following model objects are used:

Edge	Edges define the borders and the axes of structural components. Depending on their properties, they can also define a line support, a <i>beam series</i> or <i>free beams</i> (e.g. columns). Edges are normally used to describe the whole structure in a wire frame model.
Wall	Like edges, walls define the border of a structural component as well as a line support. The geometry of a component is determined by the wall axes. Walls are specifically suited for creating pure slab systems.
Hole	Holes define openings in the element mesh. They can also define line supports and continuous beams according to their properties.
Column	Columns define a point support or, alternatively, a column head condensation in the element mesh.
Face	Faces describe areas that are automatically meshed by the mesh generator. Edges, walls and holes form the outer border of these meshes. All model objects inside the face are taken into consideration during mesh generation. The face can be assigned various properties for the FEM mesh.
Cone	The <i>Cone</i> model object describes a truncated cone segment for subsequent meshing. You can use the shortcut menu to determine the degree of intersection with other selected cones or model faces. A cone can also be assigned various properties for the FE mesh.
Solid	The model object <i>Solid</i> describes a solid body for the subsequent mesh generation with tetrahedron elements. Properties for the FE mesh can be assigned to the solid.

Model objects have no further significance once the element mesh is generated. However, they should be saved as drawings for reuse at a later time.

For an area to be meshed, a closed polygon must be defined using edges and/or walls. If you want to mesh multiple areas in different planes at the same time, then model faces should be defined as well.

Selecting

If you want to edit or delete an object, you first need to select it. This can be done in a number of different ways:

- 1. Select individual objects via mouse click. You can expand or reduce the selection by holding down the *Shift* or *Ctrl* key while clicking the objects.
- 2. Select an area by holding down the mouse button and dragging the cursor. When you let go of the mouse button, all objects completely inside the drawn rectangle will be selected.

Selecting with the right mouse button, the shortcut menu with the standard commands for the selected objects will appear.

- 3. The *Select* command in the *Edit* menu allows consecutive additions/deletions of the selection using selection boxes for example.
- 4. The *Select group...* command in the *Edit* menu allows you to select objects based on shared properties such as *color*, *layer* or *section*.



5. The Select All command in the Edit menu.

All of these selection methods can be combined with one another. Selections are removed when you click on an empty space in the workspace.

You can access the shortcut menu for objects that are already selected by holding down the Shift or Ctrl key and pressing the right mouse button.

Entering Coordinates

You can enter coordinates with either the keyboard or the crosshair.

If you are using the keyboard, enter the x, y and then z coordinates separated by a space in the dialog bar. The measures are always meter [m]. The positive direction corresponds to the displayed coordinate system.

Start of edge:	Arc	Circle	Properties	1.25 2.5	Enter	Esc
I×						^
ý						

For 2D editing you only need to enter the first two coordinates. The program will automatically set the third coordinate to zero. The decimal separator (e.g., on the numeric keypad) conforms with the one defined in the regional settings.

Global coordinates	These coordinates always refer to the global zero point.
Relative coordinates	Die These coordinates refer to the last point that was entered (local zero point). A small axis system is always visible at this point. To activate relative coordinate input, click the <i>Relative</i> button.

Using the crosshair to enter coordinates, it automatically refers to an existing object.



Example 1: Slab with Downstand Beam

This example shows you how to specify and calculate a basic floor plate and then process the results.



A new project is started when you launch the program or select *New* from the *File* menu. Choose the *Finite Elements* structure type from the *structure* menu.

Drawing Ground Plans with Model Objects

Ground plans and in spatial structures wire frame models are initially described using the Edge, Column and Hole model objects. These objects will later be recognized by the mesh generator and taken into account during mesh generation.

You do not need to make any definitions regarding the area:

Simply draw your static system. The mesh generator will take care of the rest.

Meaning:	Support:	<u>P</u> itch;	Angle [°]
Support 🗸 🗸	Jointed	~	
Beam series Free beam	31		Joints
Pile beam		~	Eccent
Spring Standard			Show
Support Support+Beam se	eries	×	√ symbol (is valid for all objects)
Layer of elements	:	\underline{C} olor of elements:	Partial View el.:
Take over	~	Take over 🗸 🗸	Take over 🖂

For Meaning set the option *Support* and leave the dialog with *OK*.

A crosshair will appear in the representation area and you will be prompted to enter the coordinates of where the edge starts in the dialog bar. Enter [0 0] to position the start of the edge at the origin. After you press [\downarrow] or Enter, the end of the edge will be queried.

R To enter additional coordinates, you should activate relative coordinate input by clicking the *Relative* button. A small axis system will now always appear at the last point inserted. All successive coordinates you enter always refer to this point. Enter [0 2.1] to define the end of the edge in the positive y direction. If you enter consecutive coordinates, the subsequent edge ends will always be queried. The remaining coordinates are therefore:

[-4.9 0] [0 -6.3] [4.9 0] [0 -2.8] [7 0] [0 3.5] [3.5 0] [0 4.2] [-4.9 0] [0 -2.1]

🕸 Untitled.fem - InfoCAD				-		>
<u>File Edit View Format Draw</u>	Snap Structure Options	Analysis Help	-			
	X ΩΩ ⊠1:	✓ € [†]	🛼 - 🔪 A O A 🔲 🍃 🛄 🖷		D +16+	ŝ
h _{ap} h‰ ⊲ ≥ ⊐ -¢ 76- r-p ≯	a 🖌 • i 📲 🖀 🏹	🗣 🖓 👯 🎬 🖉 🗛 🕭	±ଯ়¢ ଔ ६ ≣ ⊯ № №	\$… # ∓ .	🖻 🗇	
Data Base 📮 🗵	End of edge: Start	Arc Properties		Enter	Esc	1
🖃 🙆 Untitled.fem	×					^
🗉 🛄 Display	y y					
Drawings Structure Description						
Load cases						
🛞 🧰 Predeformation		ø	B			
E Load Case Combination						
BS EN 1992-1-1 Design		8				
EN 1995-1-1 Steel Checks EN 1995-1-1 Timber Checks				,		
Dynamics						
💿 🧰 Prestressed Concrete						
표 🚞 EN 1992-2 Bridge Checks			×	60		
	6					
			le l			
		ø	_			
		_				
						~
					-	_
	Output				ф	×
🞑 Dat 🎥 Pri 🗗 Par 🔁 Layer						
Ready			5,6000 -1.400	0.0000	CAP	NUN

To temporarily enlarge the view, click Zoom - or Zoom all.

Q

You can begin entering another edge without having to use the command over again by selecting the *Start* option. Now enter the missing edges. In the process you can click on the endpoints of existing edges.



B

Click the *Column* button and enter the column midpoint. To move the current reference point for the entry of coordinates, click the *Basepoint* button and select the end of the edges to the right of the column as the new reference point. Confirm the *Old* or *Global* setting (the local reference system is not to be rotated) as the *New direction of x axis.*

The column should be 3.5 m in the negative x direction from the selected edge end point, which means the coordinates for the column midpoint are: [-3.5 0].



- +16+ One way to check your input is to dimension the system. To do so, click the *Dimension* button and the *Objects* option. Now select *all* of the specified model objects. Dimension points are the end points of all objects. For example, to position a horizontal dimension chain, select *Horizontal* and click the drawing area below your system.
- Click *Save* to store all the data you have entered up to this point. The Windows file dialog will appear and prompt you to enter a file name. The *.*fem* extension will be assigned automatically. Your drawing will also be given a name (E.g. Ground_plan) and is saved in this file.

Specifying Supports and Cross-Sections

The properties of the specified Edge and Column model objects are preset as follows:

Edge: Support jointed

Column: Support jointed

The correct support conditions are thus already set for this system.



To modify the properties of the area elements, select the *Element Properties* command. Click *New* and then change the section type to *Area* for **Section 1**. The thickness is preset correctly with 0.2 m and the material must be selected to *C20/25-EN*. For the next input this material is preset.

The default settings for the remaining section properties such as bedding, reinforcing steel and EN 1992-1-1 are correct for this example.

Click *New* again and generate **Section 2**. Change the section type to *Polygon*. The input window for graphically describing the polygon is displayed:



T

You can generate and dimension the polygon shown above using the *Downstand Beam* command. Close the window by clicking *OK*.

Activate the Reduce dead load button to decrease the dead load of the T-beam by the portion attributed to the slab.



Beam series (downstand beams) with the specified section should be generated at the two last entered edges to replace the supports. Open the *Edge* dialog by double-clicking the edge you want to modify. Now change the *Meaning* to *Beam series* and select Section 2. You do not need to specify the section at the end of the edge.

Generate the FEM Mesh

The finite element mesh can be generated automatically using the specified model objects. To do so, click the *Mesh Generation* button and select the *Form-sensitive* option. Arrow indicators will now appear marking the outer area limits recognized by the program. Specify the mesh width (0.45 in this example) to generate the FEM mesh, supports and beam series.

The drawing is not required anymore. Deactivate the drawing in the database 🗹 📧 Ground_Plan

Defining Load Cases

Open the Load cases folder in the database and double-click Wew load case. A new load case will be created automatically and the load input and editing buttons will appear in the dialog bar.

Click the Load creation button to open the New Load dialog:



Select *Dead load* with a weighting of 1 in the Z direction and then click *Apply*. Using this load type, the dead load will be generated automatically based on the sections and material. This load is indicated by the *Dead Load* label in the upper right corner of the image.

Now select *Uniform region* on the *Area load* tab and select two points to specify a rectangle that surrounds the entire slab. The load ordinate q_z should correspond to 1.5 kN/m². This load will be applied to all elements located entirely within the area.

Click *Change Number* to modify the load case number or to specify a load case name.

Create the new load case 2 using the function *New load case*. Select the *Uniform rectangle* area load under the *Load creation* option. The load rectangle is now represented by three coordinates. Click on the three corner points of the load field. In this case the load ordinate is $q_{z} = 3 \text{ kN/m}^2$:



Create the next load case and use the Uniform polygon area load for each remaining field.

Nr.

<u>*</u>

Trace the next field and then click *Close* and enter the load ordinate $q_z = 3 \text{ kN/m^2}$. The program will then apply triangle areas to the defined load area.

Now enter the depicted load cases in consecutive order, each time selecting *New load case*:



Specifying Actions and Design Situations

×

When carrying out the design, first the internal forces of the actions are combined. The program uses cyclic permutation to analyze all variations of the leading and accompanying actions. The safety factors and combination coefficients are automatically taken into account as specified by EN 1990. The extremal internal forces are then used to determine the design values.

This is done by simply assigning the existing load cases to the actions:

- G Dead load: Load case 1
- QN Imposed load, traffic load: Load case 2-5

Open the \square EN 1992-1-1 design folder in the database and select the \square Actions submenu item. A dialog will appear where you can define actions in accordance with EN 1992-1-1. The program suggests a new G Dead load action. Confirm by clicking OK.

Now assign the relevant load cases to the action by selecting a load case in the left pane and moving it to the *Selected load* cases pane on the right:

G Dead load	×
Label: Dead load	Gamma.sup: Gamma.inf:
Load cases: <u>M</u> ulti-select	Selected load cases:
2 Area 1	1 Dead load
3 Area 2	
4 Area 3	
5 Area 4	<< ≥> OK Cancel

Exit the Dead load dialog and create a new QN Imposed load, traffic load action.

New	action(s) ×
G	Dead load
GE	Earth pressure
GH	Permanent hydrostatic pressure
P	Prestressing
QN	Imposed load, traffic load
QS	Snow and ice load
QW	Wind load
OT	Temperature action
	OK Cancel

Introductory Examples

Assign the remaining traffic loads by moving them to the Inclusive load cases area.

QN Imposed load, traffic load		×	QN Imposed load, traffic load			×
Label:	Gamma.sup:	Gamma.inf:	Label:	Gamma.sup:	G <u>a</u> mma.ir	nf:
Imposed load, traffic load	1.5	0	Imposed load, traffic load	1.5	0	
Combination coefficients psi fo Superstruc	tures	\sim	Combination coefficients psi fo Superstruct	ures	~	
Working load - category A: Residential bu	ildings	~	Working load - category A: Residential buil	dings	~	
Load cases: Multi-select	Exclusive variar	nts: 🗶 🛪	Load cases: Multi-select	Exclusive varia	ints:	% 1
2 Area 1	1. Variant			1. Variant	~	
3 Area 2	Inclusive load c	ases: X		Inclusive load	cases:	X
4 Area 3 5 Area 4 ≤< ≥>				2 Area 1		
				3 Area 2		
				4 Area 3		
				5 Area 4		
	Exclusive load o	ases:		Exclusive load	cases:	
<< >>			<< >>			
	ОК	Cancel		ОК	Cance	

Inclusive load cases	Selected load cases that can have a simultaneous effect.
Exclusive load cases	Selected load cases that are mutually exclusive (e.g., wind from right / left or individual mobile load positions)

The action QN is now defined and you can exit the dialog.

Next click *Situation...*, select *Permanent and temporary situation* and then accept the default settings. The *Rare (characteristic) situation* is required to define the robustness reinforcement. Create this as well.



The actions and situations are now defined. You can edit an action or situation by double-clicking it. The user-friendly tree structure provides an easy way of checking the included load cases.

Click OK to exit the EN 1992-1-1 actions dialog.

Performing Calculations

You can now perform the following calculations:

- Statics analysis (determines deformations, internal forces, support forces, etc., for each load case)
- EN 1992-1-1 design (bending and shear design)

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The calculations can be started automatically in consecutive order. To do so, click the *Batch...* button and select the relevant check boxes.

Batch	×
 ✓ Statics Analysis Dynamics Analysis Load Case Combination ✓ EN 1992-1-1 Design EN 1992-2 Bridge Checks EN 1993-1-1 Steel Checks EN 1995-1-1 Timber Checks Nonlinear Analysis 	OK Cancel

If necessary, you can control the calculation and logging process by clicking the Calculation settings... menu item.

The progress of the calculation and any errors or warnings will be displayed on the output bar.

Processing Results

All of the available results are listed in the Results folder in the database. Double-click a result to open the available or designated standard view (graphic or table) of that result.

For instance, open the *Node deformations* results folder and then double-click *Load case 1*. The deformed system will be displayed. To see the deformation figure, switch to the *3D view*.

To change the viewing angle you can use the *Rotate with mouse* button.





Click the *Results* button in the results bar to open a dialog that lets you select the display option and settings for all results:

Results		×
Category Set	tings	
Representat.:	Deformation figure \sim	
Scale Auto small Display <u>o</u> rigi	Colored Deformation figure Isolines Numeric Section Result units nal system in dashed manner	-
Label - Off \v Decimal places: 2 \v	$\begin{array}{c c} \text{Limit value for} \\ \text{labeling:} \\ \text{Height } \geq : & \leq [mm]: \\ \hline 2 & \hline \hline \end{array} \end{array}$	-
	OK Cancel Help	

If you select representation *Section*, you can click the *Define section* button in the results bar and define the sections for the results view.

Please note that some view options are not available depending on the category you selected:

- A deformation figure can only be displayed for all components simultaneously and not for an individual displacement component (e.g., uz).
- The min/max line from an internal forces combination of area elements can only be displayed numerically or in the section.

EN 1992-1-1 Checks

In addition to the bending and shear design that has already been carried out, you can also perform additional checks in the ultimate limit state and the serviceability limit state:

- Bending with or without normal force or normal force only (EN 1992-1-1, Chapter 6.1).
- Minimum reinforcement against failure without warning (Chapter 5.10.1 (5)P and 9.2.1.1).
- Lateral force (Kapitel 6.2).
- Torsion and combined stressing (Kapitel 6.3).
- Punching shear (Kapitel 6.4).
- Fatigue of longitudinal reinforcement, shear reinforcement and prestressing steel (Chapter 6.8.5, 6.8.6)
- Fatigue of concrete under compressive stress (Chapter 6.8.7)
- Fatigue of the concret compressive struts under lateral force and torsion (Chapter 6.8.7 (3))
- Limiting the concrete compressive stresses (EN 1992-1-1, Chapter 7.2).
- Limiting the reinforcing steel stresses (Chapter 7.2).
- Limiting the prestressing steel stresses (Chapter 7.2).
- Decompression check (Chapter 7.3.1).
- Minimum reinforcement for crack width limitation (Chapter 7.3.2).
- Crack with calculation (Chapter 7.3.4).
- Limiting deformations (Chapter 7.4).

Additional input is required in the EN 1992-1-1 design folder for this purpose:

Bactions: Define the necessary design situation depending on the additional checks and the exposure class.

To do this, click New: Situation... in the action dialog and create the required situations.

New situation(s)	×	EN 1992-1-1 actions	×
Permanent and temporary situation Accidental situation Earthquake situation Rare (characteristic) situation Frequent situation Quasi-continuous situation OK Can	~ v	Standard design group Actions Standard design group Or G Dead load ON Imposed load, traffic load Or Design situation Or Design situation Or I. Rere (characteristic) situation Or I. Frequent situation Or I. Frequent situation Or I. Action Or Standard design group	OK Cancel New: Group Action Situation Edit Delete
		Use combination rules of EN 1990 (6.10a/b)	Calculate

Settings related to the section are made in the Element properties dialog. Click the *Element properties* button to open this dialog and then enter the check specifications required for each section under *EN 1992-1-1*.

Section	Number: Section Type:	м	aterial Type: Now Co
···· Form	1 - Slak V Area V		20/25-EN
Material	Label:		Delete
Default values	Label.		
Creep coefficients	SIBD		
Beinforcoment	Prestress of component:	Exposure class:	
Event 1992-1-1	Not prestressed V	XC4 V	
Checks	Life limit state	Estimus	
Base values			teal Down has show
Stresses	Bend, long. force	Reinf., prestr. s	steel Concr. long. stres
···· Crack width	✓ Lateral force	Lateral force	Concr. comp. stru
Fatigue	Iorsion	Iorsion	
····· Variation coefficients	Robustness		
	Shear joint		
BS EN 1992-1-1 Thermal products	Serviceab. limit state		
lointe	Concrete comp. stress		
Joints	Decompression	Reinf. tensile st	ress
	Minimum crack reinf.	Crack width	
nent Properties - EN 1992-1-1	1 - Crack width	ОК	Cancel Help
nent Properties - EN 1992-1- - Section Form Material	I - Crack width Number: Section Type: 1 - Slat V Area V	OK C	aterial Type: New Co
nent Properties - EN 1992-1- Section Material	I - Crack width Number: Section Type: 1 - Slat Area Label:	OK C	Cancel Help Interial Type: New 220/25-EN Delete
nent Properties - EN 1992-1- Section - Form - Material - Default values - Creep coefficients	I - Crack width Number: Section Type: I - Slat ~ Label: Slab	ок С	Cancel Help aterial Type: :20/25-€N ✓ Delete
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nent Properties - EN 1992-1- Section Form Material Default values Creep coefficients Bedding Reinforcement Reinforcement Reinforcement Reinforcement Reinforcement	I - Crack width Number: Section Type: 1 - Slat Area Label: Slab Slab Settings for cross-section er wmax: gr,max:	OK M Calculation of coeff. kc:	Cancel Help aterial Type: Ne <u>w</u> ⊆o 220/25-EN ✓ Delete Bar diameter, bar spacing max gg [rm]: max g [rr
ent Properties - EN 1992-1- Section - Form - Default values - Creep coefficients - Bedding - Reinforcement - EN 1992-1-1 - Checks - Base values - Streepse	I - Crack width Number: Section Type: I - Slat \rightarrow Area \rightarrow Label: Slab Statings for cross-section er wmax: gr,max: 0.3	OK M Calculation of coeff, kc: auto	Cancel Help aterial Type: Ne <u>w</u> <u>Co</u> 220/25-EN → <u>Delete</u> Bar diameter, bar spacing max ds [mm]: max g [m 10
ent Properties - EN 1992-1- Section Material Default values Creep coefficients Bedding Reinforcement Building Reinforcement Base values Stresses Crack width	I - Crack width Number: Section Type: 1 - Slat Area Label: Sab Satisfies for cross-section end wmax: gr,max: 0.3	OK M Calculation of Calculation of coeff. kc: auto	Cancel Help Laterial Type: New Co :20/25-€N ∨ Delete Bar diameter, bar spacing max ds [nm]: max s [n]
Anternal - Form - Form - Material - Default values - Creep coefficients - Bedding - Reinforcement - Reinforcement - Checks - Base values - Stresses - Crack width - Fatigue	I - Crack width Number: Section Type: I - Slat Area Label: Slab Settings for cross-section et wmax: gr,max: 0.3 Minimum reinforcement	OK M Calculation of Calculation of coeff. <u>k</u> c: auto	Cancel Help Jaterial Type: New Co 120/25-EN ∨ Delete Delete Bar diameter, bar spacing max gs [m]: max gs [m] 10 Crack width limitation
ent Properties - EN 1992-1- Section Form Material Default values Creep coefficients Bedding Reinforcement EN 1992-1-1 Checks Base values Stresses Crack width Fatigue Variation coefficients	I - Crack width Number: Section Type: I - Slat Area Label: Slab Stabe: Settings for cross-section end wmax: gr,max: 0.3 Iminimum reinforcement Determ. of the tensile gone: Determ. of the tensile gone:	OK M dge: Standard ~ Calculation of coeff. kc: auto	Cancel Help aterial Type: New Qo 20/25-EN Delete Bar diameter, bar spacing max ds [mm]: max g (m 10 Crack width limitation Check combination, method
ent Properties - EN 1992-1- Section Form Default values Creep coefficients Bedding Reinforcement EN 1992-1-1 Checks Base values Crack width Fatigue Variation coefficients E) FN 1992-2	I - Crack width Number: Section Type: I - Slat Area Label: Slab Settings for cross-section er wmax: gr,max: 0.3 Iminum reinforcement Determ. of the tensile gone: Comb acc. to class	dge: Standard ✓ Calculation of coeff. kc: auto △Ac,eff	Cancel Help laterial Type: New Co 220/25-EN Delete Delete Bar diameter, bar spacing max ds [mm]: max g [m] 10 Crack width limitation Check combination, methor Comb. acc. to class
Anternal Properties - EN 1992-1- Section - Form - Material - Default values - Creep coefficients - Bedding - Reinforcement - EN 1992-1-1 - Checks - Stresses - Crack width - Fatigue - Variation coefficients - Ben 1992-2-2 - BS EN 1992-1-1	I - Crack width Number: Section Type: I - Slat Area Label: Slab Slab Settings for cross-section er wmax: gr,max: 0.3 Iminum reinforcement Determ, of the tensile gone: Comb acc. to class	OK M Calculation of coeff. kc: auto Ac,eff ring-shaped	Cancel Help Jaterial Type: New Co 220/25-EN Delete Delete Bar diameter, bar spacing max ds [mm]: max g [m] 10 Crack width limitation Check combination, methor Comb. acc. to class
nent Properties - EN 1992-1- Section - Form - Material - Default values - Creep coefficients - Bedding - Reinforcement - Busser - Base values - Stresses - Crack width - Fatigue - Variation coefficients - EN 1992-1-1 - Thermal analysis	I - Crack width Number: Section Type: 1 - Slat Area Label: Sab Settings for cross-section er mmax: gr,max: 0.3 gr,max: Minimum reinforcement Determ. of the tensile gone: Comb acc. to class	OK M Calculation of Calculation of coeff.kc: auto Ac,eff ring-shaped Prestr. steel,	Cancel Help Laterial Type: New Co 120/25-EN Delete Delete Bar diameter, bar spacing max ds [mm]: max g [m] 10 Crack width limitation Check combination, mether Comb. acc. to class Direct calculation
Anternal Properties - EN 1992-1- Section — Form — Material — Default values — Creep coefficients — Bedding — Reinforcement — N 1992-1-1 — Crack width — Fatigue — Variation coefficients — BS EN 1992-1-1 — Thermal analysis Joints	I - Crack width Number: Section Type: I - Slat Area Label: Slab Settings for cross-section ed mmax: wmax: gr,max: 0.3 gr,max: Ocentral Component Determ. of the tensile gone: Comb acc. to class General Component Eactor f. fctm: Coeff. k:	OK M Calculation of Calculation of Calculation of Calculation of Calculation of Calculation of Calculation of Calculation of Prestr. steel, coeff. <u>ig</u> 1:	Cancel Help Jaterial Type: New Co 120/25-EN ∨ Delete Delete Bar diameter, bar spacing max ds [nm]: max g [nr 10 Crack width limitation Check combination, methor Comb. acc. to class Direct calculation Factor f. fgtm: Load dur.;

After you are done, start the *EN 1992-1-1 design*. The program will determine the required reinforcements, perform the checks and then generate a log.

A detailed description of all the checks is available in the chapter EN 1992-1-1 Design of the InfoCAD help and the user manual.

Printing

<u>à</u>

You always have the option of printing the current screen view. Click *Print Preview* to see what the actual printout will look like. The settings for the page format, printer selection, etc., can be modified using the *Page Setup...* command in the *File* menu.

Tables or logs need to be selected in the database and then printed out from the shortcut menu (right-click). The *Page Preview* command is also available in the shortcut menu.



🥵 Slab_s.tem - IntoCAD —	×
Print Next Page Prey Page Iwo Page Zoom In Zoom Qut Qose	
Finite Elements 21.30a x64 © InfoGraph GmbH Slab_s.fem - 12/21/2022 3:18:08 PM - Page 1 System characteristics 578 Nodes 551 Elements 22 Beams 106 Supports 529 Slabs 0 Link elements 0 Plains 2 Material properties 0 Solids 2 Section properties 0 Solids 0 LC Combinations 0 Spring elements 0 Tendon groups Result locations in beam elements Result location in area elements: Node 2 Result locations in beam elements 0 Element systems 0 Internal force systems 0 Reinforcement systems 3 Result location systems 0 Reinforcement systems 3 Result locations systems 3 Reinforcement systems	^
1 Area Slab Demonthickness [m] Orbotropy dx/dz E-Modulus slab/plain 2 0.200 Orbotropy dx/dz E-Modulus slab/plain 1 2 Polygon Downstand beam Centroid (m) Vs = 0.620 zs = 0.184 Area (m ²) 4 Area (m ²) Moments of inertia [m4] X = 3.4400e-01 Moments of inertia [m4] Vs = 3.355e-03 Vs 11 = 8.3355e-03 Vz 12 = 3.2238e-02 Dead load wthout slab part of T-beam. Averaging of the lateral force shear stress over section width Material properties No. Type E-Modu, G-Modu Poiss alpha t [1/K] [1/K]/m ²] 1 1 C20/25-EN 30000 12500 0.20 1.00e-05 25.000	

Printing List

Specified views as well as tables and logs can be added to the printing list for printing at a later time. Only display parameters are stored, meaning changes made to the system are automatically reflected in the list.

Switch from the database to the printing list. The Print and Page Setup commands can also be accessed in the printing list.

Click the *Add to printing list* button to copy the current system view into the printing list. All active settings will be saved with the system view.

Use the shortcut menu to add tables or logs selected in the database to the printing list. You can also add separate text objects, page breaks or a table of contents.

You can sort existing entries in any order you want by simply dragging them with the mouse.



Double-click entries in the printing list to open the corresponding view in the representation area.

Example 2: 2D Hall Frame

This example shows you how to specify a 2D hall frame and perform the following calculations:

- Static calculation
- EN 1993-1-1 Steel Checks (elastic; plastic at stress exceeding in classes 1 and 2)

Please select in the menu Analysis / Settings ... EN 1993-1-1 the required national annex of the standard. This will be used in the subsequent inputs and calculations.

Activate the *Elastic; plastic at stress exceeding in classes 1 and 2* - button.

Settings	×
Statics Load Case Combination EN 1992-1-1 EN 1993-1-1 EN 1995-1-1	
National edition of the standard for steel structures:	
EN 1993-1-1/AC:2009 (Basic document) V	
Verification of the resistance of cross sections for classes 1 to 4: O Elastic	
Elastic; plastic at stress exceeding in classes 1 and 2	In accordance with Chapter 6.2.1 of the standard,
O Elastic; plastic for all cross-sections with classes 1 and 2	the elastic cross-section resistance is verified for
Determine plastic limit forces for all cross sectios by equilibrium iteration	classes 1 to 4. If the comparison stress in classes 1 and 2 exceeds
Use option of the chapter 5.5.2(9) for cross sections of class 4	the permissible limit, the plastic cross-section resistance will be verified. For every set of internal
Determination of check internal forces:	forces the cross-section class is automatically
O Min/Max combination	determined.
Opplete combination	
<u>A</u> ctions <u>P</u> artial safety factors Standard ∨	
OK Cancel Help	

Task



A new project will be started when you launch the program or select *New* from the *File* menu. Choose the **2D Frame** structure type you want to use from the *structure* menu.

The representation area will switch to the x-z view and you can now start specifying the system.

Specifying the Framework

2

Click the *Element creation* button and a crosshair will appear in the representation area. You will then be prompted to enter the coordinates of the beam starting point in the dialog bar. Enter [0 0] to position the first beam node at the global origin. Press [,,] or *Enter*. Next, the beam end point will be queried.

R To enter additional coordinates, you should activate relative coordinate input by clicking the *Relative* button. A small axis system will now always appear at the last point inserted. All successive coordinates you enter always refer to this point. Enter [0 -8] to define the beam end in the negative z direction. You have now specified the first beam.



To temporarily enlarge the view, click Zoom - or Zoom all.

Enter the remaining beam ends in consecutive order and adjust the view where necessary: [15 -3] [15 3] [0 8]

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<u>File Edit View Format Draw S</u>	nap Str <u>u</u> cture <u>O</u> ptions <u>A</u> nalysis <u>H</u> elp		
	X Ω Ω ∅1:	🗐 8 🗊 +16t	
k k k k k k k k k k k k k k k k k k k	🖌 - i 📲 🖉 🗑 🖓 睤 📾 🖉 🧛 と 🖢 🞵 🗢 🖓 🐨 🐂 🛱	∓ , 🕅 🗗	
Data Base 🛛 📮 🗵	Beam end point: Start Properties Enter	r Esc	N
 Untitled.sw Structure Description Load cases Predeformation EN 1992-1-1 Design BS EN 1992-1-1 Design EN 1992-1-1 Design EN 1993-1-1 Steel Checks EN 1995-1-1 Timber Checks 		x z a	✓ ≪ ≪ ⊕ □ □ ₽ > × → ↓
💁 Dat 🍄 Prin 🚰 Parti 🔁 Layer			
Ready	30.0000, 0.0000, 0.0000	CAP	NUM

Solution will be prompted to select the beam nodes in the dialog bar. You can click the nodes directly with the crosshair or select them using the *Window* option (to specify a rectangular snap window). Press *Enter* after you have selected the nodes. Specify *Yes* when the program asks you for the global alignment.

In the Support dialog you can individually adjust each degree of freedom if necessary.

Support	×	🕸 Untitled.esw - InfoCAD — 🗆 X
<u>S</u> upport: Jointed ∨		
Displacements: Twists: x: Fix	>	Data Base Data Control Info Enter Exc N Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info Image: Control Info <t< td=""></t<>
OK Cancel		☐ Data_¶@Pint[PParti] Elayer <

Confirm that the support is set to *jointed* and exit the *Support* dialog by pressing *OK*. The supports are indicated by their corresponding icons.



Exit beam input by pressing [↓] or Enter.

Defining the Section



Click the *Element properties* button and set the section type to *HEA 550* and the material type to *S235-EN* for section 1.

Beam Properties - Section - Form	X Section Library	×
Section Material Type: Material Type: Shear stresses Material Bedding HEA 550 \$235-EN Thermal analysis Image: Section Type: Material Type: Image: Section Type: Joints Image: Section Type: Material Type: Image: Section Type:	ew Copy Delete Instructions Instructions Instrelinstructinstructions <td< td=""><td>brary</td></td<>	brary
	with international steel profiles.	

The complete framework is now defined and the corresponding parts list is available in the workspace in the Structure description folder.

List of steel profiles

	Section	Length [m]	Quanti ty	Total [m]	Weight [kg/m]	Weight [kg]	Total [t]
1 2 3	1 HEA 550 - S235-EN 1 HEA 550 - S235-EN Sum	8.000 15.297	2 2 4	16.000 30.594 46.594	166.420 166.420	1331.36 2545.74	2.66 5.09 7.75

Checking the Structure Properties

You can activate a number of different view options to check the data you have entered. To do so, click the *View options* button:



Defining Load Cases

Open the Load cases folder in the database and double-click Vew load case. A new load case will be created automatically and the load input and editing buttons will appear in the dialog bar.



Click the Load creation button to open the New Load dialog:

lew Load General Beam loads	>
Dead load Fire scenario Influence line Insert Load group Predeformation Superposition	Support displacement Temperature Temperature (initial strain) Theory
ОК	Cancel Apply

Activate the *Dead load* and click *Apply*. Using this load type, the dead load will be generated automatically based on the sections and material. This load is indicated by the *Dead Load* label in the upper right corner of the image.

Now choose *Line load* from the *Beam loads* tab, check the *Global orientation* option, select both beams of the horizontal frame member and enter a load ordinate of $q_z = 3.9$ kN/m:



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Nr.

You can adjust the load diagrams in any manner you want using the Load representation button.

Click Change Number to modify the load case number or to specify a load case name.

Create the new load case 2 by pressing $\boxed{1}$ New load case. Choose Line load from the Load creation tab, check the Projection orientation option and enter a snow load of $q_z = 5$ kN/m.



Starting with New load case, enter the following load cases:





Load case 3: Wind from the left

Load case 4: Wind from the right

The 'Load case 5: Crane load left' requires a single load to be positioned inside the frame column. This load should be applied 2 meters below the frame corner. After selecting the *Point load* load type, simply click the corresponding frame column and enter the ordinates as shown below:

Load Data	×		
Point load global (EGZ) From beam: To beam: I Distance from start (z): Px: 6 m O kN Pz: My: 100 kN -40 kNm Load-time function: OK Cance	00 40 æ	٥	

For precise positioning, define a load location with a distance of 6 meters to the beam start.

Now enter 'Load case 6: Crane load right' (distance of 2 meters to beam start) and 'Load case 7: crane load middle' accordingly, with half the load on each frame column.

EN 1993-1-1 Steel Checks

You first need to describe the actions according to EN 1993-1-1. To do so, assign different actions to the existing load cases:

- GDead load:Load case 1QSSnow and ice load:Load case 2QWWind load:Load case 3 and 4QNImposed load, traffic load:Load case 5, 6 and 7
- G Dead load × ahel Gam na.inf: 1.35 1 Load cases: Multi-select Selected load cases: 2 Snow load 1 Dead load 3 Wind from the left 4 Wind from the right $\leq \leq \geq >$ 5 Crane load left 6 Crane load right 7 Crane load middle right. ОК Cancel

Open the \square EN 1993-1-1 Steel Checks folder in the database and select the \square Actions submenu item. A dialog will appear where you can define actions in accordance with EN 1993-1-1. The program suggests a new G Dead load action. Confirm by clicking OK.

Now assign the relevant load cases to the action by selecting a load case in the left pane and moving it to the *Selected load cases* pane on the right.

Exit the *Dead load* dialog and create a new *QN Imposed load, traffic* load action. Assign the remaining traffic loads by moving them to the *Exclusive load cases* area.



Inclusive load cases	Selected load cases that can have a simultaneous effect.
Exclusive load cases	Selected load cases that are mutually exclusive (e.g., wind from right / left or individual mobile load positions)

Do the same with the QS Snow and ice load and the QW Wind load action.

Next click Situation..., select Permanent and temporary situation and then accept the default settings.



The actions and situations are now defined. You can edit an action or situation by double-clicking it. The user-friendly tree structure provides an easy way of checking the included load cases.

Click OK to exit the EN 1993-1-1 actions dialog.

Performing Calculations

You can now start the following calculations:

• Static analysis (determines deformations, internal forces, support forces, etc., for each load case)

EN 1993-1-1 Steel Checks

(superposition of actions with calculation of determinant design values, check for elastic or plastic utilizations)

In the standard case, the checks are performed at 5 calculation points per beam. Due to the overlarge element division, it is recommended to select 11 calculation points for beams in the analysis settings for static analysis in this example.

The calculations can be started automatically in consecutive order. To do so, click the *Batch...* command in the calculation menu and activate the relevant check boxes.

The progress of the calculation and program messages, such as errors or warnings, will be displayed in the output bar.

Processing Results

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All of the available results are listed in the *Results* folder in the *database*. Double-click a result to open the available or designated standard view (graphic or table) of that result.

For instance, open the Andrew Node deformations folder and then double-click Load case 3. The deformed system will be displayed.

Click the *Results* button to access various display settings. Internal forces and comparison stresses from the *1. permanent and temporary situation*:





You can activate the calculated utilizations in the *Stresses / Steel Checks* folder. Note that the depicted frame can only be checked if the plastic internal force limits are taken into account.



LCC EN1993.SV.1: 1. Permanent and temporary situation, EN 1993-1-1 Utilization. 1.02 [-] = +----+ Value range (overall system, min/max): 0.16/0.93 [-]



In the stress view right-click any beam and select *Section Stress* from the shortcut menu.

The stresses at the section will now be applied for the selected location.

In the depicted window you can select the results for all load cases and combinations as well as for $\sigma_{x'} \sigma_{v'} \tau_{x}$, τ_{xy} and τ_{xz} stresses.

The checking program records the calculation process in a concise log. You can view the log in the 🕮 Listings folder.

Combination informat	ion			×
EN1993.SV.1: 1. Perm Internal forces Beam 4	anent an - Loc. 1,	d temporar Node 4 - N	ry situation 1y min	, EN 1993
Load case	Factor	Nx	Му	Qz
1 : Dead load	1.35	-114.91	-444.03	55.50
2 : Snow load	1.50	-112.50	-434.73	54.34
3 : Wind from the left	0.90	-4.32	-57.03	7.13
5 : Crane load left	1.05	-1.40	-17.12	2.14
My min		-233.13	-952.91	119.11
Print.		->Print list	t	ОК

The combination information shows you the individual factors contributing to an internal force result.

In the results view right-click the desired results location in the beam and select *Combination information* from the shortcut menu.

Printing

You always have the option of printing the current screen view. Click *Print Preview* to see what the actual printout will look like. The settings for the page format, printer selection, etc., can be modified using the *Page Setup...* command in the *File* menu.

Tables or logs need to be selected in the workspace and then printed out from the shortcut menu (right-click). The *Page Preview* command is also available in the shortcut menu.



🞼 2D Hall Frame EN 1993.esw - InfoCAD			×
Print Next Page Prev Page Two Page Zoom In Zoom Out Close			
Print Next Page Prev Page Two Page Zoom In Zoom Out Close 2D Frame 21.30a x64 © InfoGraph GmbH 2D Hall Frame EN 1993.esw - 12/21/2022 4:19:18 PM - System characteristics 5 Nodes 4 Beams 2 2 Supports 0 Link elements 1 Material and section properties 1 Aca cases 0 Load cases 0 Load case 0 Load case 1 Result locations in beam elements Section properties 1 1 Frainf 1 Tea Im 1 Centroid [m] 1 Ye = 0.000 1 Tea Im 1 Moments of inertia [m4] 1 Ye = 1.1190e-03 1 Last andle [Grad] 1 Nonents of inertia force shear stress over section width	Page	1	
Naterial No. Type E-Modul alphat gamma			
1 1 S235-EN 210000 1.2e-05 78.500			v
Page 1		I	IUM

Printing List

Specified views as well as tables and logs can be added to the printing list for printing at a later time. Only references to the specified display parameters are stored, meaning changes made to the system are automatically reflected in the list.

Switch from the database to the printing list. The printing list has its own Print and Page preview commands.



Use the shortcut menu to add tables or logs selected in the database to the printing list. You can also add separate text objects, page breaks or a table of contents.

You can sort existing entries in any order you want by simply dragging them with the mouse.



Double-click an entry in the printing list to open the corresponding view in the representation area.

InfoGraph Systemviewer

The InfoGraph Systemviewer allows you to visualize FEM and framework projects as well as to animate deformations from static or dynamic calculations.

Launch the Systemviewer from the *Programs / InfoGraph* folder in the *Start* menu and open the file you want to view. A variety of settings options are available to help you optimize the graphical output.



Example 3: 3D Building

This example shows you how to specify a 3D building and perform the following calculations:

- Static calculation
- Design according to EN 1992-1-1

It is assumed that you have already gone through the previous examples. Hence, some of the processing steps will not be detailed in this example.

Task



Material: C20/25-EN (Concrete EN 1991-1-1) Reinforcing steel BSt 500, axis distance of 3.0 cm from edge

Load: Dead weight, area load, horizontal load

Procedure to enter 3D structures:

- 1. Describe a wire frame model using edges
- 2. Assign the wanted properties to the edges (support, beam series, free beam)
- 3. Enter model faces
- 4. Mesh generation

Drawing the wire frame model

Launch the program or start a new project by clicking New and then selecting Finite Elements under the Structure menu.

Click the Edge button and begin by entering:

[0 0]



Activate relative coordinate input and complete the outer edge of the 1st level:

[8 0], [0 8.5], [-8 0]

Point to the start of the 1st edge to connect the lines. End the edge input with Enter or ESC.

Select the upper edge with the right mouse button and choose *Copy directly* from the shortcut menu.



The program asks for the displacement vector. Select e.g. the upper left end of the edge to define the start point. Now activate the option *Multiple*. For the second point of the displacement vector enter the coordinates [0 2] and [0 4]. The edge will be copied 2 and 4 meters in y direction. Finish the copy process with *Enter* or *ESC*.

Now switch to the 3D view and create the other levels using the copy function.

To do so, select the edges by dragging a box over all the objects. The selected objects will now be displayed with dotted lines.

Select Copy directly in the Edit menu and specify the displacement vectors:

- 1. Point of displacement vector: [0 0 0]
- 2. Point of displacement vector: [0 0 3]
- 2. Point of displacement vector: [0 0 -3]

For 3D editing, always enter 3 coordinates in the order x, y and z. Based on the above coordinates, the displacement vectors point +/-3 meters in z direction.

i If you want to temporarily change the angle of view, click *Rotate with mouse* and move the cursor while holding down the left mouse button. Click the button again to stop the rotation.



⁽a) You can adjust the view to fit the new object dimensions using the Zoom all button.

Ø

Enter the missing edges between the levels. It can be referred to the existing points with the snap modes *mid-point*, *endpoint* or *intersection*. In this way, further manual coordinate input can be avoided.



×

Select the unnecessary edges and *delete* them.

To shorten the lower edges select these edges with the right mouse button and choose the option *Intersect*. For border object select the relevant edge.



In order to create supports at the columns later we use the model object *Column* with the option *Point* at the foot points of the columns.

The wire frame model is completed.

Assigning Properties

Additional properties can be assigned to the edges to be taken into account while generating the mesh. Double-click one or more edges to define the properties and set the wanted *meaning*:

Edge X Meaning Result from the mesh g Meaning: Support: Pitch: Angle [°] Standard Area limitation or fixed lim Support Image: Jointed Image: Standard Line support on the edge Beam series Image: Joints Joints Beam series Beam elements connect Prie beam Image: Show Symbol (is Valid for all objects) Beam elements with intersection, e.g. column frames Support Image: Color of elements: Partial View el.: Support +Beam series Line support and beam series	generator ne inside
Meaning: Support: Pitch: Angle [°] Standard Area limitation or fixed lin Support Image: Jointed Image: Standard Line support on the edge Beam series Image: Joints Joints Beam series Beam elements connect Pile beam Image: Show Show Show Show Free beam Support Show Symbol (is valid for all objects) Symbol (is valid for all objects) Beam series Line support and beam series Layer of elements: Color of elements: Partial View el.: Support +Beam series Line support and beam series	ne inside
Support Image: Color of elements: Doints Beam series Image: Color of elements: Doints Support Show Support Support Support Support Support Support HBeam series Color of elements: Partial View el.:	
Joints Joints Joints Beam series Spring Show Standard Show Support Support Support Support Support Support Support Support Support Support Support +Beam series Color of elements: Beam series Support +Beam series Line support and beam series Support +Beam series	2
Spring Show Show Show Free beam Beam elements with intersection, e.g. colum frames Support +Beam series Color of elements: Eartial View el.: Support +Beam series Support +Beam series aver of elements: Color of elements: Partial View el.: Support +Beam series Line support and beam series	ted to the eams
aver of elements: <u>Color of elements</u> : <u>Partial View el.</u> : Support +Beam series Line support and beam se	n indeper ins or part
	eries
Take over V Take over V	
Y Keaning: Support: Pitch: Angle [9]	
Free beam v v 1 0 z Free beam series Free beam 1; Joints	
Link element ✓ Eccent Pile beam ✓ Eccent Spring Show Show Standard Symbol (is valid for all Valid for all	\mathbf{k}
bjects) Layer of elements: Color of elements: Partial View el.:	
	H

ý

OK

Cancel

For the columns set *Free beam* with *pitch* = 2. Start the section dialog and define the polygon section with the function \dots (e.g.: Rectangle 30x60).

ŵ

BIM-ID:

Assign the property *Support* to the supported edges and *Beam series* with the polygon section to the outer downstand beams.

Click the Face button and select the Layer and Color you want to use for the element mesh before defining the area (Properties option). Double-click inside the section window to open the section dialog. Set the properties for the area section and leave the dialog.

Model face X	Element Properties - Section - Form
Sections of elements: 2 - FL d=0.2 · Walls and slabs Rotation of the element coordinate systems [°] Int. forces: Reinforcem. Factor for mesh width: 0 0 0 1 Layer of elements: Color of elements: Bartial View of elements: 5 2 Take over Label: Eccentricity [m] 0 Selected subareas Unselected subareas	Section Number: Section Type: Material Type: New Copy Material Default values Z2/25-EN Delete Delete Bedding Reinforcement Walls and slabs Delete Delete Delete Best 1992-1-1 D: N992-2 D: Section Type: Material Type: New Copy Delete Delete
Active Inactive BIM-ID: OK Cancel	OK Cancel Help

Now select the limiting objects for the first area. An area will be created as soon as it is fully defined by the selection. Change the *color* now and again if necessary.



The geometric description of the system is now complete. Save the file and enter a name for the drawing.

Generating an Element Mesh

The model structure that is created serves as the basis for FEM mesh generation.



Click the *View options* button to access various settings for how the system is displayed. In the resulting dialog, activate the *Area fill of the elements* option to improve the 3D quality of the structure.

Check the structure using the section representation.

Defining Load Cases

<u>a</u>

Open the Load cases folder in the database and double-click **New load case**. A new load case will be created automatically and the load input and editing buttons will appear in the dialog bar.

Nr. For load case 1, select *dead load* with weighting 1 in the z direction, confirm your selection and then enter a load case label using the *Change Number* button.

Click **New load case** in the database again to create the new load case 2. Select the area load *Uniform rectangle* and define two rectangle area loads on the floors. When entering coordinates refer to existing nodes and use the functions *Zoom* and *Rotate with mouse*.





For load case 3, select Line load for area elements with global orientation.

New Load X	Load Data	×
General Point load Line load Area load Database	Line load, global (LKO+LG)	
Beam monent Beam trapezoidal load Free line load Free line moment	Starting point, ya: za: 8 8.5 -3 End point, xe: ye: ze: 0 8.5 -3 qxa: qxe: 0 0 kN/m 0 kN/m 0 kN/m	
Global orientation Local orientation Selection of lines, arcs	qya: qye: 0 kN/m 0 kN/m qza: qze: 10 kN/m 10 kN/m	
OK Cancel Apply	OK Cancel	



의 With the Load representation option you can scale the load ordinates.

For the fourth load case create two Line loads with 2.4 kN/m and one area load with 0.8 kN/m² in global x direction.



Load input is now complete and you can exit the Load dialog.

Describing Actions

EN 1992-1-1 actions	×			
Standard design group Actions G Dead load G. G Dead load G. QN Imposed load, traffic load G. QN Imposed load, traffic load G. QN Imposed load, traffic load G. QW Wind load G. QU Wind load G. QW Wind load	OK Cancel New: Group Action Situation Edit Delete Combina Calculate	Open databas create a G QN QW	Actions in the EN 1992-1 e, assign the following actions Permanent and temporary situ Dead load: QN Imposed load, traffic load: Wind load:	-1 Design folder of the s to the load cases and ation. Load case 1 (dead load) Load case 2 and 3 (area load, line load) Load case 4 (horizontal load)

Calculation and Results

Begin the *batch* calculation with the options selected as shown below.

The selected calculations are performed automatically in consecutive order. You can follow the progress of the calculations in the output bar.

Once the calculation is finished, all the results are available in the database. Open the results category you need and select the appropriate view.

Deformations load case 1

4......

■ Ø Ø Ø Ø D D D & & &

6

Esc

DEFECT

Enter

\$ **...**

_D

Colored progression of the moments max mx

Longitudinal reinforcement of the beams

Lateral reinforcement of the beams and columns

InfoGraph Systemviewer

The InfoGraph Systemviewer allows you to visualize FEM and framework projects as well as to animate deformations from static or dynamic calculations.

Launch the Systemviewer from the *Programs / InfoGraph* folder in the *Start* menu and open the file you want to view. A variety of settings options are available to help you optimize the graphical output.

Example 4: Prestressed Roof Girder

This example involves a wide-spanned roof construction that is represented as a continuous girder over two spans with a double-sided cantilever. To edit this system, perform the following work steps:

- Enter a static system .
- Enter a tendon group
- Enter the loads
- EN 1992-1-1 Design
- Display the results

The structure is constructed for exposition class XC1. In accordance with EN 1992-1-1, Table 7.1N no check of decompression is necessary for this exposition class.

It is assumed that you have already gone through the previous examples. Hence, some of the processing steps will not be detailed in this example.

Task

Static system and dimensions [m]

Material

Concrete	C45/55-EN
Reinforcing steel	BSt 500, axis distance from edge 5 cm

Section

Tendon groups

Four bundled tendons are arranged in this example. The tendon group guide is shown in the next figure. The depicted tendon group ordinates z, of the spline points refer to the upper edge of the section.

Tendon groups in beam series Tendon view 1, [-16.00/0.00/0.00] - [112.00/0.00/0.00] / [-16.00/0.00/1.00] Superelevation = 10 0 0

Tendon group ordinates zv [cm] at the base points

xv	0.00	6.40	12.80	19.20	25.60	32.00	38.40	44.80	51.20	57.60	64.00	70.40	76.80	83.20	89.60	96.00	102.40	108.80	115.20	121.60	128.00
1	70.0	52.0	19.2	42.0	120.2	180.1	208.8	201.5	157.4	78.3	18.5	78.3	157.4	201.5	208.8	180.1	120.2	42.0	19.2	52.0	70.0

Prestressing steel and prestressing system	
Prestressing steel quality	St 1500/1770
Certification of the prestressing system	EC2
Number of tendons in the bundle	4
Section surface area A_z	1800 mm²
E-modulus of the prestressing steel	195000 MN/m²
0.1% strain limit (yield strength) of the prestressing steel $f_{ m p0,1k}$	1500 MN/m²
Tensile strength of the prestressing steel $f_{ m pk}$	1770 MN/m²
Permitted prestressing force of the tendon $P_{ m m0}$	2295 kN
Prestressing loss from relaxation of prestressing steel	4.5 %
Friction coefficients when increasing or releasing strain	0,2
Unintentional angle displacement of a tendon eta '	0.3 °/m
Slippage at prestressed tie bolt	6 mm
Duct diameter	82 mm
Scattering coefficients of the internal prestressing	
Construction stage (r_{sup} / r_{inf})	1.1 / 0,9
Final state (r_{sup} / r_{inf})	1.1 / 0,9

Loads

Load case 1	Dead load (G1)
Load case 2	Additional loads 11.06 kN/m (G2)
Load case 3	Snow load 7.90 kN/m (Q)
Load case 10	Prestressing (P)
Load case 15	Creep-generating continuous load: G1+P+G2
Load case 20	Creep and shrinkage (CSR)
	Coefficients: $\varphi_{t\infty}$ = 2.55; ρ = 0.8; $\varepsilon_{cs.t\infty}$ = -24.8 \cdot 10 ⁻⁵
	Creep-generating continuous load case: 15 The redistribution of internal forces between concrete and prestressing steel are taken into account.

Static System

Launch the program or start a new project by clicking *New* and then selecting *Finite Elements* from the *Structure* menu.

To accurately measure the prestressing and the creep and shrinkage behavior, first divide the structure into beams with a length of 4 meters. Specify the supports based on a fork support of the system.

First switch to the xz view.

- Click the button *Element creation* and select the element type *RS Beam*. For automatic division activate the option *Generate*. A crosshair will appear in the representation area and you will be prompted to enter the coordinates of the starting point in the dialog bar. Enter [0 0] to position the starting point at the origin. After you press [,] or *Enter*, the end of generation line will be queried.
- R To enter additional coordinates, you should activate relative coordinate input by clicking the *Relative* button. A small axis system will now always appear at the last point inserted.
 All successive coordinates you enter always refer to this point. Enter [16 0] to define the end of the generation line in x direction.

Generate beams	×	
Section at the <u>b</u> eginning:	~	
Section at the <u>e</u> nd:	~	Enter 4 for the number of beams which shall be generate
Number: Overall len.	OK Cancel	

To temporarily enlarge the view, click Zoom - or Zoom all.

Now enter continuously the remaining section coordinates and choose the number of beams to be generated:

[48 0] - 12 Beams; [48 0] - 12 Beams; [16 0] - 4 Beams

Finish the beam input with [\dashv] or *Enter*.

Now click the *Supports* button and choose the *Point support* option. You will be prompted to select the support nodes in the dialog bar. You can click the nodes directly with the crosshair.
 During the support input take care to support the system free of restraint in the longitudinal direction and to define a

Entering a Tendon Group

fork support.

The definition of the tendon groups does not depend on the elements. The assignment and force introduction onto the structure occur while the *Prestressing* load case is being calculated. To avoid any conflicts, a distinction is made between beam and area/solid prestressing.

The tendon curve on which the calculation approach is based is represented by a 3D cubical spline function. The spline function is the curve that runs through all specified spline points with the least amount of curvature. The basis for the prestressing analysis is provided by a tendon group force curve that allows for strain increase, release and slippage. To include prestressing in the FEM analysis, you need to define a load case with the *Prestress* load type.

	-	-	_	
-		5	20	

Die Use the *Prestressing* function to enter and edit tendon groups.

Tendon group properties	×
General Prestressing System Prestressing Procedure Number: Label: Prestressing system 1 Unnamed 1 - Unnamed Number of tendons: Iype: Beam Prestressing proced 1 Image: I	 Click <i>Define</i> and then select <i>Beam</i> for the tendon group typ in the resulting dialog. Normally, the spline points are defined graphically.

Tendon group properties X	
General Prestressing System Prestressing Procedure	
Number, Label: Certification: 1-EC2	
Area Ap : fp0,1k: E-Modulus: 1800 mm² 1500 MN/m² 195000 MN/m² fpk:	The <i>Prestressing System</i> tab contains all the properties that are assigned to the tendon groups. They apply to each individual tendon of the group
1770 MN/m² 2295 kN 82 mm Friction coefficients µ Unintentional Tension: Release: Slippage: angular disp. ß': 0.2 0.2 6 mm 0.3	
OK Cancel	
Tendon group properties X	
General Prestressing System Prestressing Procedure	If the allowance value K is used, then the factor for over-
I - Example Kappa:	stressing refers to the maximum permitted force \boldsymbol{P}_{\max}
Normalized1. Ten- sioning1. Re- lease2. Ten- sioning2. Re- leaseStart:1000End:1000	Using the factor specified for the release, the maximum prestressing force remaining in the tendon group is defined with respect to $P_{\rm m0}$. A factor of '0' means no release.
OK Cancel	

Exit the dialog with OK and enter the spline points of the tendon group (start, supports, midspans) at the nodes of the beam array.

When you are done, click *Enter*. The tendon group no. 1 is now positioned along the centroidal axis of the beam array.

Additional editing should take place in a section view. To access a section view, select the entire beam series with the *View... / Beam* option and define the view plane from the first node to the last node in the global z direction.

Tendon Group View		×	
Views: [Standard view] Tendon view 1 Visible tendon groups: 1 Unnamed	Beam Remaining t >> OK	Representat.: Height zv V Superelevation: 10 Text height: Text height: tendon groups:	Add tendon group 1 to the <i>Visible tendon groups</i> list.

Reactroscod Roof Girdor form InfoCAD																		_		~
· Prestressed Koor Griderhenr - InfoCAD																				~
<u>F</u> ile <u>E</u> dit <u>V</u> iew Forma <u>t</u> <u>D</u> raw <u>S</u> n	ap Str <u>u</u> cture <u>(</u>	Options	<u>A</u> nalysis	<u>H</u> elp																×
🗅 🖆 🖬 🎒 🔃 🕺 🖻 🛍 🗡	< _ _ \Z	1: System	n ·	~ 🖯	fi - 1	■ 📮	a - 🔨	A C	γ	2		17 E		181	₱ +16†	2	•	\times	$\downarrow \vdash \bigcirc$	R B
N ₁₀ NS ≪ ▷ 의 → X4 ep X4		霍 勹	I 🖓 🖓	막 114	[4] 3 ^[1]	8	↓ <i>1</i> 7 +	₩ 🖉 🖡	*	##	at I ju	577 \$	 ∰ ≥	7 -	6) 🖬 •				
Data Base 📮 🗵	Prestressing:	Close	Save	•	Define	S	oline points	View.	💌	Zero	point	Base	points				E	nter	Esc	N
Prestression control of the first inter- Drawings Structure Description Drad cases Dradeformation Drade Case Combination Drade Case Combination EN 1992-1-1 Design EN 1993-1-1 Steel Checks Dramics Dynamics	zv	Tendon g Superelev 0	roups in vation = 1	beam s 10	series Te	endon	view 1, [-	16.00/0.	00/0.00	ı] - [11	2.00/0).00/0.	.00] / [-1	5.00/0.0	10/1.00)]		< 1		く e e e e e e e e
																				ť×
	ler	idon grou	p ordinat	es zv [cmjatt	he bas	e points		-						_					z
	xv 0.	00 6.40	12.80 19	20 25.6	32.00	38.40	44.80 51	.20 57.60	64.00	70.40	76.80	83.20	89.60 9	6. 00 102.4	40 108.	80 115.20	121.60	128.00		z'
	1 52	.5 52.5	52.5 52	2.5 52.	.5 52.5	52.5	52.5 53	2.5 52.5	52.5	52.5	<mark>52.5</mark>	52.5	52.5 5	2.5 52.	.5 52.	.5 52.5	52.5	52.5		÷
🔯 Data, ♀⊖ Print 🖓 Parti 📚 Laver	<																			× -
Ready	,														-16.0	000. 0.0	000. 0.0	000	U	E NUM .

The tendon group is now displayed in the selected section. Next, use the *Zero point* option to move the position of the reference system from the centroid to the upper edge of the section and then in the *Base points* option select *Spline points*.

If the tendon group is selected, you can reposition it by first clicking the existing spline point and then the *Spline points* option. Only change the respective Zv position. The *Insert* option allows you to add additional spline points before the active spline point.

Use this method to assign the desired geometry to the tendon group.

xv	0.00	6.40	12.80	19.20	25.60	32.00	38.40	44.80	51.20	57.60	64.00	70.40	76.80	83.20	89.60	96.00	102.40	108.80	115.20	121.60	128.00
1	70.0	52.0	19.2	42.0	120.2	180.1	208.8	201.5	157.4	78.3	18.5	78.3	157.4	201.5	208.8	180.1	120.2	42.0	19.2	52.0	70.0

In the tendon group view, the coordinates always refer to the reference system.

Click the Representation option to activate the resulting prestressing force curve in the tendon group view:

Loads and actions according to EN 1992-1-1

Entor	tha	following	loads.
Enter	une	DIIOVVIIIQ	IOdus.

- Load case 1 Dead load (G1)
- Load case 2 Additional loads11.06 kN/m (G2)
- Load case 3 Snow load 7.90 kN/m (Q)
- Load case 10 Prestressing (P)
- Load case 15 Creep-generating continuous load: G1+P+G2

Load cases 1, 2 and 10 are grouped into this load case with the *Insert* load type.

Lastfall 20 Creep and shrinkage (CSR)

Coefficients: $\phi_{t\infty}$ = 2.55; ρ = 0.8; $\epsilon_{cs.t\infty}$ = -24.8 \cdot 10⁻⁵

The *Creep and Shrinkage* load type lets you calculate the redistribution of internal forces between the concrete and prestressed steel. Specify load case 15 as a creep-generating continuous load case.

The following checks are carried out as a part of this example:

Checks in the Ultimate Limit States

- Bending with or without normal force or normal force only
- Lateral force
- Minimum reinforcement against failure without warning

Checks in the Serviceability Limit States

- Limiting the concrete compressive stresses
- Limiting the reinforcing and prestressing steel stresses
- Minimum reinforcement for crack width limitation with f_{ctm} = 1; k = 1; ξ_1 = 0.27
- Crack with calculation

The following input is required for these checks:

- Select and adjust the settings for the checks in the element properties
- Define the actions

To carry out the desired checks the following situations are necessary:

- Permanent and temporary
- Rare (characteristic)
- Frequent

t_o:

t∞:

To take different (construction) stages into account, three situations are defined:

Construction stage: Dead load G and prestressing P (tendon not grouted)

Dead load G, P, additional load and snow load (tendon grouted)

Dead load G, P, additional load, snow load and CSR (tendon grouted)

1. Permanent and temporary situation - Structural cond. X	2. Permanent and temporary situation - t0 X	3. Permanent and temporary situation - t∞ ×
Label:	Label:	Label: Einal state Construction stage Actions: G Dead load G Additional dead load P Prestressing CSR1 Creep, shrinkage, relaxation Q S Snow and ice load
OK Cancel	OK Cancel	OK Cancel

Calculation and results

Perform the following calculations after all system specifications have been made:

- Statics
- DIN EN 1992-1-1 Design

The necessary design situations are used for all checks depending on the requirement class. Each situation is checked independently and the maximum reinforcement for each steel layer is stored.

A number of different calculation results are shown below:

Internal forces

LCC EN1992.MAX: Maximum EN 1992-1-1

Bending reinforcement, steel layer z >= zs; z < zs (dashed). 52.63 [cm²] = ⊢

Value range (overall system, min/max): 0.00/89.82 [cm2], total weight from design: 5.5 t

Shear reinforcement

Value range (overall system, min/max): 5.37/13.96 [cm²/m]

Concrete compressive stresses

LCC EN1992.C: Maximum, Rare (characteristic) situation, EN 1992-1-1 Concrete compressive stresses min,max Sigma.x. 9.89 [MN/m²] = Value range (overall system, min/max): -16.88/2.69 [MN/m²]

Reinforcing steel stresses

LCC EN1992.C: Maximum, Rare (characteristic) situation, EN 1992-1-1 Reinforcing steel stresses min,max Sigma.s. 58.30 [MN/m²] = Value range (overall system, min/max): -99.50/43.29 [MN/m²]

Prestressing steel stresses

LCC EN1992.C: Maximum, Rare (characteristic) situation, EN 1992-1-1 Prestressing steel stresses min,max Sigma.p. 709.29 [MN/m²] = Value range (overall system, min/max): 936.03/1210.52 [MN/m²]

LCC EN1992.C: Maximum, Rare (characteristic) situation, EN 1992-1-1 Prestressing steel stresses Sigma.p/Sigma.p,perm. 0.56 [-] = + Value range (overall system, min/max): 0.82/0.95 [-]

Excerpt from the 'EN 1992-1-1 Design' log (short version)

Design of longitudinal reinforcement

The calculated requ. reinforcement includes the specified basic reinforcement.

 (M) Nominal reinf. for robustness as per EN 1992-2, 6.1 (109) (Charact. C.)
 (R) Nominal/requ. reinforcement as per 7.3.2/4 for crack width limitation Increase of reinforcement due to crack width check is marked by "!". Part of prestr. steel area Xil*Ap which was used to reduce req.As Bond coefficient for prestressing steel as per Eq. (7.5) Ap' Xil

(B) Design of reinforement at ultimate limit state Results from a column design using the method with nominal curvatures according to 5.8.8 are marked with "+". In case of dominant bending, compression reinforcement is marked with "*". For section areas acc. to 6.1 (5) the conrecte strain is not limited. The minimum reinforcement acc. to 9.2.1.1 and 9.3.1.1 is not determined. For compressive members the minimum reinf. acc. to 9.5.2 is considered. whereby the prestressing steel area of bonded tendons is taken into account.

Beam			Rein	forcem	ent Nx	My	Mz	Ap '	req.As
No.	Se.	Lo.	Lay.	Type	[kN]	[kNm]	[kNm]	[cm²]	[Cm 2]
17	1	1	1	М	109,56	-11704,67	0,00		44,91
				R	-6042,73	-9624,61	0,00		15,38!
				B	-6714,14	-16871,48	0,00		18,11
			2	М	109,56	-11704,67	0,00		44,91
				R	-6042,73	-9624,61	0,00		15,38!
				B	-6714,14	-16871,48	0,00		18,11
			3	М	0,06	-6609,23	0,00		0,00
				R	0,00	0,00	0,00		0,00
				В	-7555 , 93	-4040,19	0,00		0,00
			4	М	0,06	-6609,23	0,00		0,00
				R	0,00	0,00	0,00		0,00
				В	-7555 , 93	-4040,19	0,00		0,00

Design of shear reinforcement

The percentage of nominal reinforcement acc. to Eq. (9.5N) is considered.

VRd, TRd	Design ง	value of maxin	num abso	orbable latera	l ford	ce, torsi	lonal mor	nent			
Angle	Angle co	ot Theta betwe	een the	compressive s	trut a	and the b	beam axis	3			
Asb,Asl.T Req. stirrup reinf. from lateral force and torsion, torsional reinf.											
	Exceddir	ngs as per Eq.	. (6.12)	are marked b	y "!"						
Asl	Req. lor	ngitudinal rei	inf. acc	. to Fig. 6.3	for 1	req. Asb.					
Beam	Qy/	Asb.y	Qz/	Asb.z	Asl	Q/VRd+	Asb.T	Asl.T			
No. Lo	c. VRd	Angle[cm²/m]	VRd	Angle[cm²/m]	[Cm 2]	Mx/TRd	[cm²/m]	[Cm ²]			
17 1	0,00	2,50 0,00	0,55	2,50 13,96							

Check of crack widths

The check is led by direct calculation of the crack width. The final long. reinforcement as the maximum from robustness, crack and bending reinf. incl. a possible increase resulting from the fatigue check is decisive.

wk Calculated value of crack width as per 7.3.4 [mm]
wmax Permissible crack width as per specification [mm]
Sigma.c Maximal concrete edge stress in state I [MN/m²]
 (CC) Charact. (rare), (TC) Frequent, (QC) Quasi-continuous combination

Bea	m				Reinf	. Nx	My	Mz	Sigma.x	wk	wmax
	No.	Se.	с.	Lo.	Layer	[kN]	[kNm]	[kNm]	[MN/m²]	[mm]	[mm]
	17	1	TC	1	1 ·	-6042,73	-9624,61	0,00	1,96	0,07	0,20

Check of concrete compressive stress

For the check, a cracked concrete section (II) is assumed if the tensile stress from the decisive c. exceeds the value of fctm. Otherwise, a non-cracked section (I) is used. If the strain is not absorbable on cracked section, (I^*) is marked.

Sigma.x,minTotal maximal longitudinal compressive stress [MN/m²]Sigma.x,per= 0.60*fck for Charact. C. (CC) as per 7.2 (2)(t,b)Position of the edge point: above, below of centreBeamSigma.x,minNo. Se. Loc.[MN/m²]11(I)-16,88-27,009xFinalCC.3

Check of steel stress

For the check, a cracked concrete section is assumed. For tendon groups without bond and/or for situations before grouting, the prestressing steel stress is checked acc. to Eq. (5.43).

Type S Long. reinf. from N and M, layer number, Charact. C. (CC) Type P Prestressing steel, Tendon number, Charact. C. (CC) Sigma.s,per = 0.80 * fyk resp. 1.0 * fyk (CK) as per 7.2 (5) Sigma.p,per = 0.75 * fpk as per 7.2 (5)

Beam			Ste	el	As	Sigma.s	per.	Situation	
No	. s	e.	Lo.	Type	No.	[cm²]	[MN/m²]	[MN/m²]	
1	7	1	1	S	1	44,91	43,29	400,00	CC.3
				S	2	44,91	43,28	400,00	CC.3
				S	3	0,00		400,00	CC.1
				S	4	0,00		400,00	CC.1
				P	1	72,00	1049,44	1275,00	CC.1

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